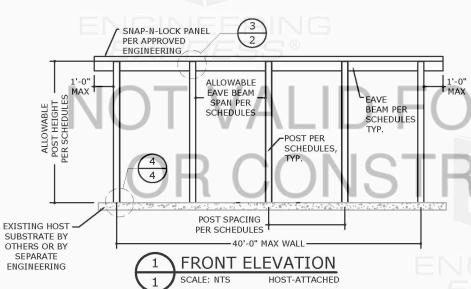
INSULATED ROOF PATIO COVER WITH OPEN WALLS

PERFORMANCE EVALUATION FREESTANDING OR HOST ATTACHED, UP TO 12' & 15' SPAN NOTE: THIS DOCUMENT IS NOT TO BE USED WITHOUT AN ORIGINAL PEN SIGNATURE & RAISED SEAL OR ELECTRONICALLY VERIFIABLE ELECTRONIC SIGNATURE MEETING ALL DISCLAIMERS SET FORTH HEREIN. RUBBED PENCIL COPIES ARE NOT PERMITTED FOR USE IN ANY WAY

FOR PERMI

SPACE RESERVED FOR CERTIFYING ENGINEER'S DIGITAL OR PHYSICAL SEAL & DATE OF CERTIFICATION

THIS IS A NON-SITE-SPECIFIC PERFORMANCE EVALUATION. A DESIGN PROFESSIONAL SHALL BE RESPONSIBLE FOR CERTIFYING THE APPLICATION OF THIS INFORMATION TO ANY SITE-SPECIFIC LOCATION



 $\binom{3}{2}$ SNAP-N-LOCK PANEL PER APPROVED ENGINEERING ALLOWABLE EAVE BEAM MAX SPAN PER BEAM PER **SCHEDULES SCHEDULE** POST PER **SCHEDULES** 4 POST SPACING EXISTING HOST PER SCHEDULES SUBSTRATE BY SEPARATE FRONT ELEVATION

EXISTING HOST EAVE SUBSTRATE BY OTHERS, TYP. 15'-0" MAX ROOF CLEAR SPAN MAX

SNAP-N-LOCK PANEL

PER APPROVED

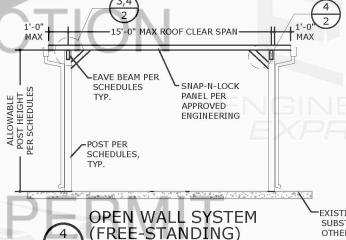
ENGINEERING

POST PER SCHEDULES **OPEN WALL SYSTEM** EXISTING HOST -SUBSTRATE BY (HOST ATTACHED)

SCALE: NTS

-EAVE BEAM PER

SCHEDULES



EXISTING HOST SUBSTRATE BY OTHERS OR BY SEPARATE ENGINEERING

POSITIVE AND NEGATIVE DESIGN PRESSURES CALCULATED FOR USE WITH THIS SYSTEM SHALL BE DETERMINED BY OTHERS ON A JOB-SPECIFIC BASIS IN ACCORDANCE WITH THE STRUCTURAL REQUIREMENTS OF THE FLORIDA BUILDING CODE 8TH (2023) EDITION, 2012/2015/2018/2021 IBC/IRC, AS WELL AS CURRENT VERSIONS OF THE MN, NC, NJ, NY, OH, SC, & VA BUILDING CODES AS APPLICABLE. CODE ENFORCED COMPLIES WITH STATE OF SEAL AND IF MULTIPLE VERSIONS LISTED THEN MOST STRINGENT APPLIES.

DESIGN SHALL UTILIZE ASD DESIGN METHOD USING ASCE 7-22 OR ASCE 7-16 BASED ON APPLICABLE CODE.

CONTRACTOR SHALL INVESTIGATE AND CONFORM TO ALL LOCAL BUILDING CODE AMENDMENTS WHICH MAY APPLY. DESIGN CRITERIA OR SPANS BEYOND STATED HEREIN MAY REQUIRE ADDITIONAL SITE SPECIFIC SEALED ENGINEERING.

SEISMIC DESIGN SHALL BE CONSIDERED WHEN REVIEWING FOR EACH USE USING LOAD TABLE LIMITATIONS PROVIDED.

THE EXISTING HOST STRUCTURE MUST BE CAPABLE OF SUPPORTING THE LOADED

ENCLOSURE AS DETERMINED BY OTHERS OR BY SPECIAL ENGINEERING. NO WARRANTY IS CONTAINED HEREIN.

THIS STRUCTURE SHALL REMAIN OPEN (NO SCREENS OR WALLS) WITHOUT ADDITIONAL ENGINEERING.

GENERAL NOTES:

STRUCTURE SHALL BE FABRICATED IN ACCORDANCE WITH ALL GOVERNING CODES. CONTRACTOR SHALL INVESTIGATE AND CONFORM TO ALL LOCAL BUILDING CODE AMENDMENTS WHICH MAY APPLY.

THE ARCHITECT/ENGINEER OF RECORD FOR THE PROJECT SUPERSTRUCTURE WITH WHICH THIS DESIGN IS USED SHALL BE RESPONSIBLE FOR THE INTEGRITY OF ALL SUPPORTING SURFACES TO THIS DESIGN WHICH SHALL BE COORDINATED BY THE PERMITTING CONTRACTOR

ALUMINUM MEMBERS ANCHORS SHALL BE SPACED WITH 2xDIAMETER END DISTANCE AND 2.5xDIAMETER MIN. SPACING TO ADJACENT ANCHORS, UNLESS NOTED OTHERWISE.

ALL CONCRETE ANCHORS SHALL BE INSTALLED TO NON-CRACKED CONCRETE ONLY THE CONTRACTOR IS RESPONSIBLE TO INSULATE ALL MEMBERS FROM DISSIMILA MATERIALS TO PREVENT ELECTROLYSIS.

ALL ALUMINUM SHALL BE 6063-T6 ALLOY AND TEMPER UNLESS NOTED OTHERWISE

ALL CONCRETE TO REACH A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI IN 7 DAYS.

CONNECTIONS:

ALL FASTENERS TO BE #12 OR GREATER SAE GRADE 5 UNLESS NOTED OTHERWISE FASTENERS SHALL BE CADMIUM-PLATED OR OTHERWISE CORROSION-RESISTANT MATERIAL AND SHALL COMPLY WITH "SPECIFICATIONS FOR ALUMINUM STRUCTURES" SECTION J.3.7.2 BY THE ALUMINUM ASSOCIATION, INC., & ANY APPLICABLE FEDERAL, STATE, AND/OR LOCAL CODES.

ANCHORS SHALL BE INSTALLED IN ACCORDANCE WITH MANUFACTURERS' RECOMMENDATIONS. MINIMUM EMBEDMENT SHALL BE AS NOTED HEREIN. MINIMUM EMBEDMENT AND EDGE DISTANCE EXCLUDES STUCCO, FOAM, BRICK, AND OTHER WALL FINISHES.

ENGINEER SEAL AFFIXED HERETO VALIDATES STRUCTURAL DESIGN AS SHOWN ONLY. USE OF THIS SPECIFICATION BY CONTRACTOR, et. al. INDEMNIFIES & SAVES HARMLESS THIS ENGINEER FOR ALL COST & DAMAGES INCLUDING LEGAL FEES & APPELLATE FEES RESULTING FROM MATERIAL FABRICATION, SYSTEM ERECTION, & CONSTRUCTION PRACTICES BEYOND THAT WHICH IS CALLED FOR BY LOCAL, STATE, & FEDERAL CODES & FROM DEVIATIONS OF THIS PLAN.

THE PRODUCT DETAILED HEREIN IS GENERIC AND DOES NOT PROVIDE INFORMATION FOR A SPECIFIC SITE. FOR SITE CONDITIONS DIFFERENT FROM THE CONDITIONS DETAILED HEREIN, A LICENSED PROFESSIONAL SHALL PREPARE SITE EXCEPT AS EXPRESSLY PROVIDED HEREIN, NO ADDITIONAL CERTIFICATIONS OR AFFIRMATIONS ARE INTENDED.

ALTERATIONS, ADDITIONS, OR OTHER MARKINGS TO THIS DOCUMENT ARE NOT

PERMITTED AND INVALIDATE THIS CERTIFICATION.

12. EXCEPT AS EXPRESSLY PROVIDED HEREIN, NO AFFIRMATIONS ARE INTENDED ADDITIONAL CERTIFICATIONS OR

VISIT ECALC.IO/STRUCTALL

& MORE INFORMATION ABOUT THIS DOCUMENT OR SCAN THIS QR CODE

VISIT ENGINEERINGEXPRESS.COM/STORE FOR ADDITIONAL PLANS REPORTS & RESOURCES



POSTAL ADDRESS: 2234 NORTH FEDERAL HWY #7664 BOCA RATON, FL 33431 ENGINEERINGEXPRESS.COM

INC.

STRUCTALL BUILDING SYSTEMS,

350 BURBANK RD OLDSMAR, FL 34677

INSULATED PATIO COVER OVER OPEN WALL ROOM SYST PERFORMANCE EVALUATION (813) 855-2627

ROOF

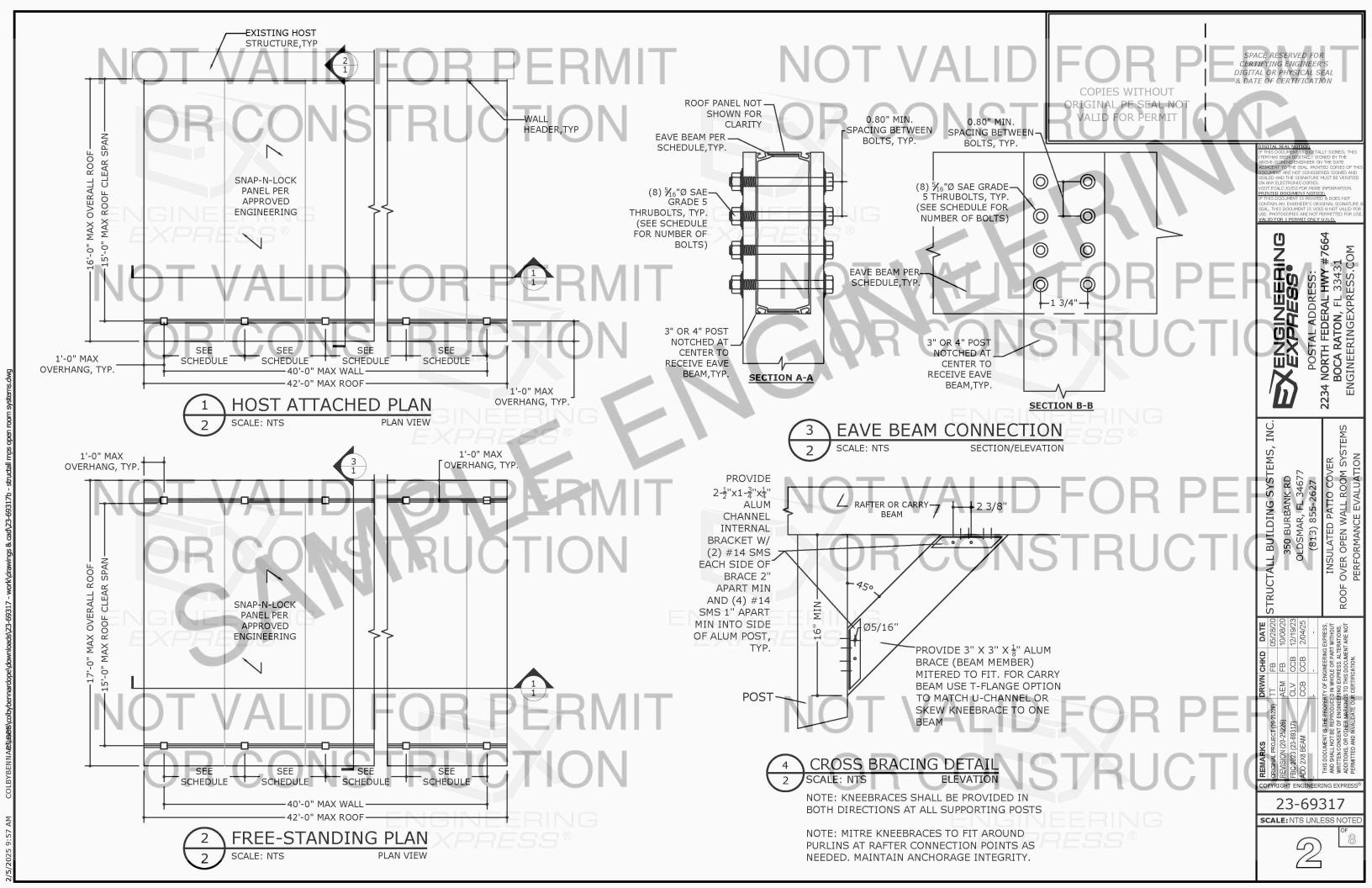
23-69317 SCALE: NTS UNLESS NOTE

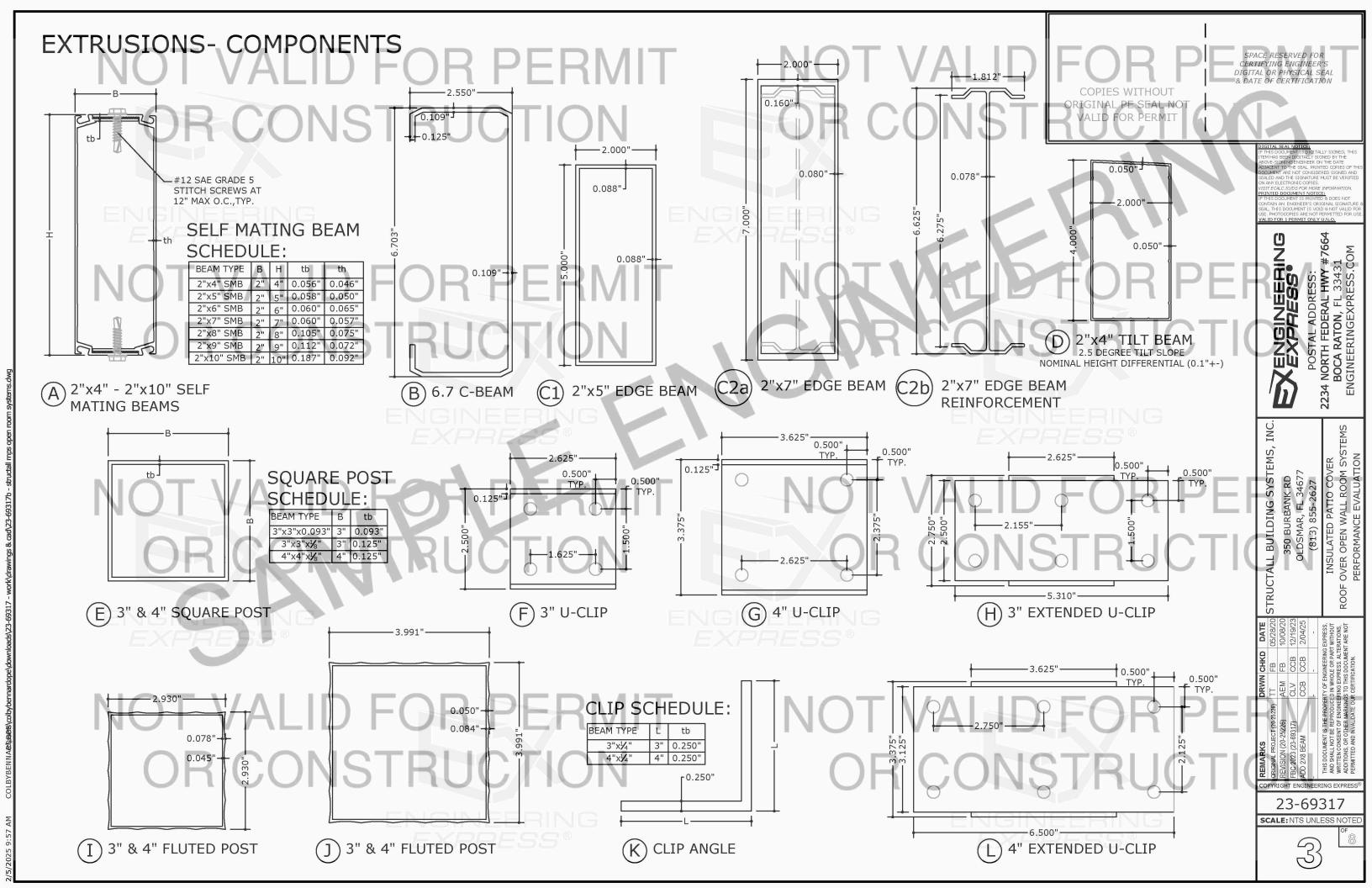


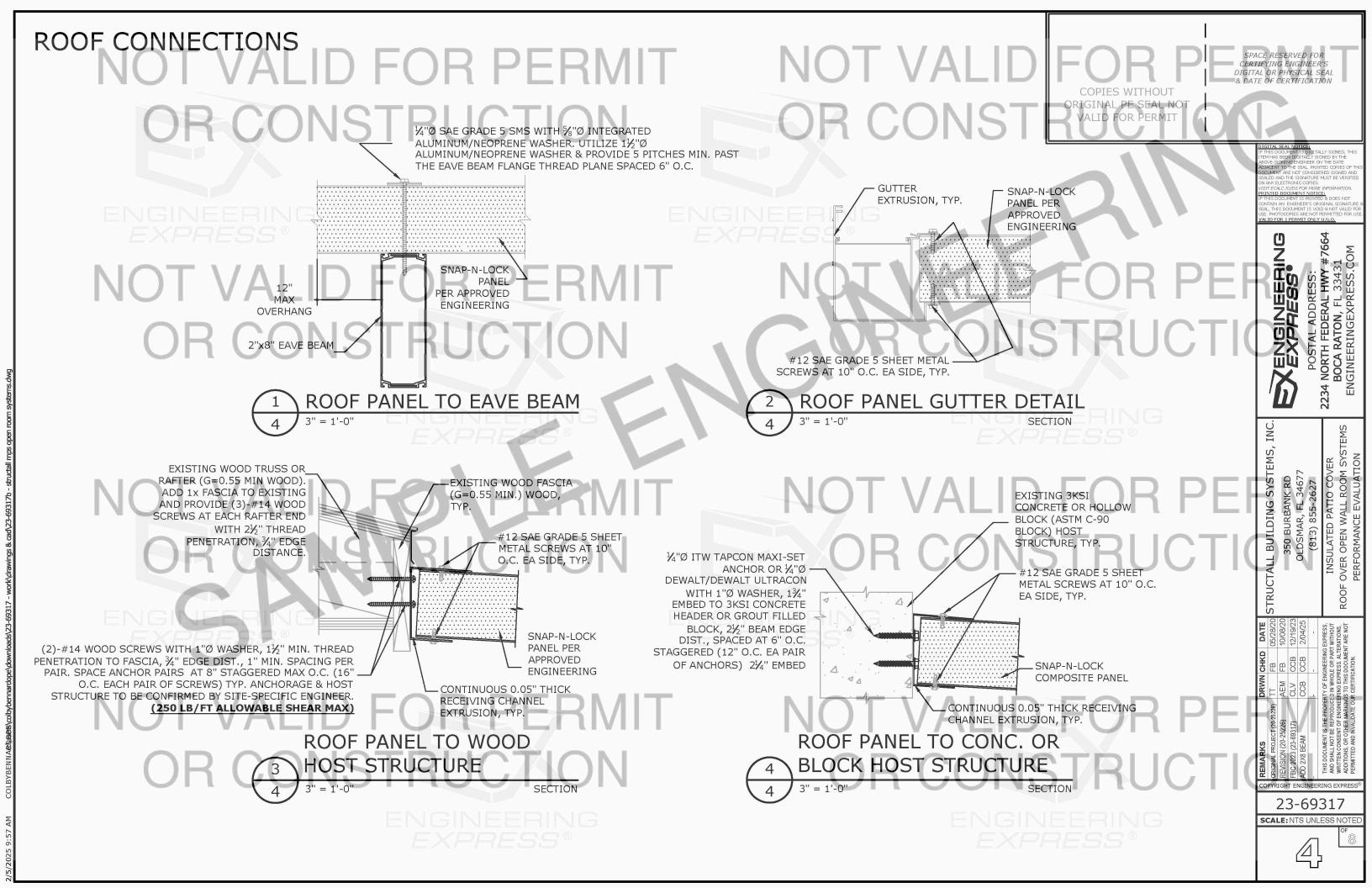
OTHERS OR BY

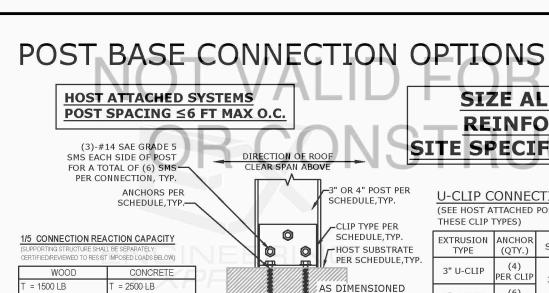
SEPARATE

FOR ENGINEER CERTIFIED ORIGINALS









ON SHEET 3

NOTE: U-CLIP CONNECTION

EXTRUSION MAY VARY (3" U-CLIF

SHOWN HERE FOR CLARITY)

-3" OR 4" POST PER

SCHEDULE, TYP

-EQUAL LEG

EXTRUSION

SCHEDULE, TYP

-ANCHORS PER

TYPE PER

U-CLIP CONNECTION

DIRECTION OF ROOF

CLEAR SPAN ABOVE

8" FOR 4" POSTS

ALUMINUM ANGLE

 $(2)\frac{3}{8}$ Ø SS BOLTS

ASTM A500 GRADE B

ASTM A36

STEEL PLATE

TIE-DOWN CONNECTION

SIZE ALL FOOTINGS & REINFORCEMENT PER SITE SPECIFIC REQUIREMENTS

U-CLIP CONNECTION DIRECTIVE:

(SEE HOST ATTACHED POST BASE SCHEDULE FOR APPLICABILITY OF

EXTRUSION TYPE	ANCHOR (QTY.)	SUBSTRATE	ANCHOR DESCRIPTION
3" U-CLIP	(4) PER CLIP	3KSI MIN.	1/4"Ø DEWALT ULTRACONS WITH ¾"Ø MIN WASHER, 1¾"
3" U-CLIP (EXTENDED)	(6) PER CLIP	CONC.	EMBED, 4" EDGE DISTANCE, 1½" MIN. SPACING.
4" U-CLIP	(4) PER CLIP	WOOD	1/4"Ø LAG SCREWS WITH ¾"Ø MIN WASHER, 2½" MIN.
4" U-CLIP (EXTENDED)	(6) PER CLIP	(G=0.55 MIN)	THREAD PENETRATION, ¾" EDGE DISTANCE, 1½" MIN. SPACING.

3/5 CONNECTION REACTION CAPACITY

(SUPPORTING STRUCTURE SHALL BE SEPARATELY CERTIFIED/REVIEWED TO RESIST IMPOSED LOADS BELOW)

WOOD	CONCRETE
The second secon	T = 2750 LB V = 3000 LB

TO CALCULATE YOUR SITE-SPECIFIC UPLIET FORCE, MULTIPLY THE NET UPLIFT FORCE X TRIBUTARY AREA FOR EACH POST HOLDS DOWN. WHICH INCLUDES HALF OF THE SPAN PLUS FULL OVERHANG. CONSULT WITH A DESIGN PROFESSIONAL IF AT ALL IN QUESTION OR USE THE MOST CRITICAL DESIGN CRITERIA & STRONGEST CONNECTION

ALUMINUM TIE-DOWN ANGLE DIRECTIVE (SEE HOST ATTACHED /FREE STANDING POST BASE TIE-DOWN

SCHEDULE FOR APPLICABILITY OF THESE CLIP TYPES)

EXTRUSION TYPE	ANCHOR (QTY.)	SUBSTRATE	ANCHOR DESCRIPTION
(2)-3"x3"x ½ " ANGLES	1 PER ANGLE (2 TOTAL)	3KSI MIN.	%"Ø HILTI KWIK BOLT 3 WITH 1½"Ø WASHER, 3½" EMBED, 9" MIN. EDGE DISTANCE, 4.95" MIN SPACING FOR 3" POSTS
(4)-3"x3"x ¼ " ANGLES	1 PER ANGLE (4 TOTAL)	CONC.	AND 5.95" MIN SPACING FOR 4" POSTS, TYP.
(2)-4"x4"x ½ " ANGLES	1 PER ANGLE (2 TOTAL)	WOOD (G=0.55 MIN)	3/8"Ø LAG SCREWS WITH 1½"Ø MIN WASHER, 2¾" MIN. THREAD PENETRATION, ¾" EDGE DISTANCE, 4.95" MIN.
(4)-4"x4"x ½ " ANGLES	1 PER ANGLE (4 TOTAL)		SPACING FOR 3" POSTS AND 5.95" MIN. SPACING FOR 4" POSTS, TYP.

MAXIMUM DIMENSION STUB POST SIZE B (IN) OF LONG THICKNESS CANOPY (FT 0.125 0.125 20

0.375 WELDED MOMENT-RESISTING STEEL BASE PLATE, ALTERNATIVE TO POST EMBEDMENT IN CONCRETE FOOTING

- WHERE APPLICABLE. WELDED POST BRACKET MUST BE FABRICATED IN ACCORDANCE FBC/IBC SECTION 1704.2.5.1 BY AN APPROVED FABRICATOR TO THE SATISFACTION OF THE CODE OFFICIAL
- MAXIMUM DIMENSION OF LONG SIDE OF CANOPY TO COMPLY WITH MAXIMUM ALLOWABLE SPANS IN MASTER
 - POST HEIGHT = 10 FT & BEAM DEPTH 8 IN, HOST ATTACHED CANOPY VALID UP TO 150MPH EXPOSURE 'C'

ADDITIONAL ENGINEERING REQUIRED BEYOND THIS WIND LIMIT.

2/5 CONNECTION REACTION CAPACITY

(SUPPORTING STRUCTURE SHALL BE SEPARATELY CERTIFIED/REVIEWED TO RESIST IMPOSED LOADS BELOW)

WOOD	CONCRETE
= 2000 LB	T = 4500 LB
/= 600 LB	V = 3000 LB

TO CALCULATE YOUR SITE-SPECIFIC UPLIFT FORCE MULTIPLY THE NET UPLIFT FORCE X TRIBUTARY AREA FOR EACH POST HOLDS DOWN. WHICH INCLUDES HALF OF THE SPAN PLUS FULL OVERHANG. CONSULT WITH A DESIGN PROFESSIONAL IF AT ALL IN QUESTION OR USE THE MOST CRITICAL DESIGN CRITERIA & STRONGEST CONNECTION.

+ EQ. + EQ. -

FOR 3" POSTS

TIE-DOWN CONNECTION

OR 4" POST PER

SCHEDULE, TY

CLIP TYPE PER

SCHEDULE, TYP

AS DIMENSIONED

ON SHEET 3

HOST SUBSTRATE

PER SCHEDULE, TYP.

DETAIL

8" FOR 4" POSTS

ALUMINUM ANGLE

DIRECTION OF ROOF

CLEAR SPAN ABOVE

NOTE: U-CLIP CONNECTION EXTRUSION MAY VARY (3" U-CLIP SHOWN HERE FOR CLARITY)

HOST

SUBSTRATE

SCHEDULE,

(2)3"Ø SS

SPACED AT

3", WITH 1

#" FDGE

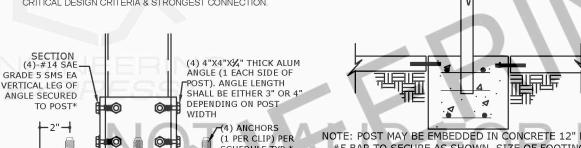
DISTANCE

ANCHORS PER

SCHEDULE, TYP.

THRU

BOLTS



DETAIL

NOTE: POST MAY BE EMBEDDED IN CONCRETE 12" MIN WITH 12" #5 BAR TO SECURE AS SHOWN. SIZE OF FOOTING & ACTUAL APPLICATION PER SITE SPECIFIC CONDITIONS BY OTHERS.

SUBSTRATE	ANCHOR DESCRIPTION
3KSI MIN. CONC.	¾"Ø HILTI KWIK BOLT 3 WITH 1½"Ø WASHER, 3½" EMBED, 9" MIN. EDGE DISTANCE, 4.95" MIN SPACING FOR 3" POSTS AND 5.95" MIN SPACING FOR 4" POSTS, TYP.
WOOD (G=0.55 MIN)	3/8"Ø LAG SCREWS WITH 1½"Ø MIN WASHER, 2¾" MIN. THREAD PENETRATION, ¾" EDGE DISTANCE, 4.95" MIN. SPACING FOR 3" POSTS AND 5.95" MIN. SPACING FOR 4" POSTS, TYP.

NOTE: THIS TABLE IS APPLICABLE FOR POST INSTALLATIONS WITH POST RIBUTARY AREAS LESS THAN OR EQUAL TO 169 SQUARE FEET

U-CLIP CONNECTION DIRECTIVE:

(SEE HOST ATTACHED POST BASE SCHEDULE FOR APPLICABILITY OF

IESE CLIP TYP	PES)		
EXTRUSION TYPE	ANCHOR (QTY.)	SUBSTRATE	ANCHOR DESCRIPTION
3" U-CLIP	(2) PER CLIP	ЗК S I MIN. CONC.	1/4"Ø DEWALT ULTRACONS WITH ¾"Ø MIN WASHER, 1¾" EMBED, 4" EDGE DISTANCE, 1½" MIN. SPACING.
4" U-CLIP	(2) PER CLIP	WOOD (G=0.55 MIN)	1/4"Ø LAG SCREWS WITH ¾"Ø MIN WASHER, 2½" MIN. THREAD PENETRATION, ¾" EDGE DISTANCE, 1½" MIN.

HOST ATTACHED SYSTEMS POST SPACING ≤6 FT MAX O.C.

4/5 CONNECTION REACTION CAPCITY (SUPPORTING STRUCTURE SHALL BE SEPARATELY

WOOD	CONCRETE					
T = 1250 LB V = 250 LB	T = 2000 LB V = 1000 LB					

TO CALCULATE YOUR SITE-SPECIFIC UPLIFT FORCE, MULTIPLY THE NET UPLIFT FORCE X TRIBUTARY AREA FOR EACH POST HOLDS DOWN. WHICH INCLUDES HALF OF THE SPAN PLUS FULL OVERHANG CONSULT WITH A DESIGN PROFESSIONAL IF AT ALL IN QUESTION OR USE THE MOST CRITICAL DESIGN CRITERIA & STRONGEST CONNECTION

NOTE: FOR FREESTANDING CONFIGURATIONS INSTALLED WITH WIND VELOCITY REQUIREMENTS >= 165MPH, EXPOSURE 'C' WITH BOTH PANEL & BEAM SPANS GREATER THAN 12 FEET (OR POST TRIBUTARY AREA GREATER THAN 169 SQUARE FEET):

JTILIZE ⅙" HILTI HIT-ICE+HAS ANCHORS (1½"Ø WASHER) WITH 4¾" EMBEDMENT, 7 MIN EDGE DISTANCE AND SPACED ACCORDING TO DETAIL 1/5. ATTACHMENTS OF CLIP LEGS TO POST SHALL UTILIZE (5) #14 SAE GR 5 SMS PER CLIP. THESE CONFIGURATIONS ARE APPROVED FOR INSTALLATIONS TO 3KSI CONCRETE ONLY, ALTERNATE SUBSTRATE INSTALLATIONS WITH THESE SPANS SHALL BE DESIGNED ON A SITE SPECIFIC BASIS.

SPACE RESERVED FOR CERTIFYING ENGINEER'S DIGITAL OR PHYSICAL SEAL & DATE OF CERTIFICATION

Y #7664 31 .COM

POSTAL ADDRESS:
234 NORTH FEDERAL HWY #
BOCA RATON, FL 33431
ENGINEERINGEXPRESS.CC

-3467 2627

INSULATED PATIO COVER OVER OPEN WALL ROOM SY: PERFORMANCE EVALUATION 350 BURBANK RD OLDSMAR. FI 24577

STRUCTALL BUILDING SYSTEM

23-69317

SCALE: NTS UNLESS NOT



WELDED STEEL BASEPLATE

CONTINUOUS T

WELD. SEE

TABLE FOR

SIZE FEX

= 60 KSI

V = 300 LB

V = 1000 LB

TO CALCULATE YOUR SITE-SPECIFIC UPLIFT FORCE, MULTIPLY THE NET UPLIFT FORCE X TRIBUTARY AREA FOR EACH POST HOLDS DOWN, WHICH INCLUDES HALF OF THE

PROFESSIONAL IF AT ALL IN QUESTION OR USE THE MOST

(4)-#14 SAE GRADE 5

ANGLE SECURED TO:

POST PER SCHEDULE,

SMS EA VERTICAL LEG OF

1" FOR 3" ANGLE

2" FOR 4" ANGLE

VARIES

PFR

SCHEDULE

HOST SUBSTRATE

SCHEDULE.TYP.

NOTE: EXTRUSION &

NUMBER OF ANGLES MAY VARY (3"x3"x1/4"

ANGLE SHOWN HER

FOR CLARITY)

SPAN PLUS FULL OVERHANG, CONSULT WITH A DESIGN

CRITICAL DESIGN CRITERIA & STRONGEST CONNECTION

HOST ATTACHED

SYSTEMS POST

SPACING > 6 FT

MAX O.C.

3.5" EMBED, 6" MIN SPACING, 6" MIN. EDGE DISTANCE, INTO MIN 3000 PSI CONCRETE

0

Ø GS HILTI KWIK BOLT 3,

ALLOWED ON MAX.

SIZE D (IN

CONNECTION CHECKED FOR PURE BENDING

ANALYSIS FOR MAXIMUM FREESTANDING CANOPY SIZE TAKING 115 MPH Vult WIND SPEED, EXPOSURE 'C',

					MAXI	мим соци	MN SPACIN	G (FT)	
COLUMN TYPE	MAX ROOF SPAN S	GRAVITY/UPLIFT ASD DESIGN LOAD	WIND LOAD	6'-0"	8'-0"	10'-0"	12'-0"	14'-0"	15'-0"
III	(FT)	(PSF)	(PSF)		ALI	OWABLE PO	ST HEIGHT	(FT)	
			20 PSF	8'-5"	4'-5"				1
			30 PSF	6'-10''	Y	(2)	-	-2	2:
		10 PSF	40 PSF	6'-0"			-	-	
			50 PSF	5'-4"	7.3	-	-	1-	-
			20 PSF	- /	-	-	-	-	.=:
			30 PSF	1 8 4	B		-	-	-
		20 PSF	40 PSF		-	1=1	-	-	
	0000	ENIC	50 PSF			-		-	-
	12'-0"	T-1-21-C	20 PSF	-				-	-
			30 PSF		(C.) (C	(R)	-	-	
		30 PSF	40 PSF	TALE		-	-		-
£ C			50 PSF	-	-	141	-	-	-
1(40 PSF	20 PSF	-		-			-
Geo			30 PSF	1.	7 - 1	-	1.	7	-
pra			40 PSF	3./			1. 1	-	-
Ď.			50 PSF	-V	//	\ .			-
ost	_		20 PSF	6'-4''		-		-	
3"x3" Fluted Post (Unbraced) 10ft			30 PSF	5'-2"	-	-	-	-	-
ute		10 PSF	40 PSF	4'-6"					
Ε.			50 PSF] - [7 . "				-
EX.			20 PSF	-	1 11		/ F 3		1
m			30 PSF					W .	J .
		20 PSF	40 PSF	-	_	-	-	-	-
	451.60		50 PSF	-	-	181	-	1-	
	15' -0 "		20 PSF		-	(5)	-		.50
		20.005	30 PSF	-	ě	-	9	-	8
		30 PSF	40 PSF	±:	_	1-1	-	-	-
			50 PSF	*	-	181	-	-	*
			20 PSF	-	-	181	-		-
		40 DCE	30 PSF		-	(5)	-		.51
		40 PSF	40 PSF	*	÷	-	-		+
			50 PSF	-	=	141	-		-

			\		MAXI	мим соги	MN SPACING	G (FT)	
COLUMN TYPE	MAX ROOF SPAN S	GRAVITY/UPLIFT ASD DESIGN LOAD	LATERAL ASD WIND LOAD	6'-0"	8'-0"	10'-0"	12'-0"	14'-0"	15'-0"
	(FT)	(PSF)	(PSF)		ALL	OWABLE PC	ST HEIGHT (FT)	
			20 PSF	10'-0"	10'-0"	10'-0"	10'-0''	10'-0''	10'-0'
		10.000	30 PSF	10'-0"	10'-0"	10'-0"	10'-0''	10'-0"	9'-6"
		10 PSF	40 PSF	10'-0''	10'-0"	10'-0''	9'-9''	8'-8''	8'-3"
			50 PSF	10'-0"	10'-0"	10'-0"	8'-9"	7'-9''	7'-4"
			20 PSF	10'-0"	10'-0"	10'-0"	10'-0"	8'-9"	8'-0"
		20 005	30 PSF	10'-0"	10'-0"	10'-0"	8'-7"	7'-2"	6'-6"
		20 PSF	40 PSF	10'-0"	10'-0"	9'-0"	7'-5"	6'-2"	5'-8"
	401.00		50 PSF	10'-0"	9'-10"	8'-1"	6'-8"	5'-6"	5'-0"
	12'-0"	30 PSF	20 PSF	10'-0"	10'-0''	9'-9"	7'-3"	5'-3"	4'-4"
			30 PSF	10'-0"	10'-0"	8'-0"	6'-0"	4'-3"	
			40 PSF	10'-0"	9'-2"	6'-11"	5'-2"		-
¥			50 PSF	10'-0"	8'-2"	6'-2"	4'-7''	181	-
11		40 PSF	20 PSF	10'-0"	10'-0"	6'-9''	4'-0''		-
4"x4" Fluted Post (Unbraced) 10ft			30 PSF	10'-0"	8'-4''	5'-6"	-	-	9
bra			40 PSF	10'-0''	7'-3"	4'-10''	2		-
į.			50 PSF	9'-5"	6'-6"	4'-3"	-	-	_
ost		1.0	20 PSF	10'-0"	10'-0"	10'-0"	10'-0"	10'-0"	10'-0'
9			30 PSF	10'-0"	10'-0"	10'-0"	10'-0"	9'-5"	8'-10'
uţe		10 PSF	40 PSF	10'-0"	10'-0"	10'-0"	9'-3''	8'-2"	7'-8"
표			50 PSF	10'-0"	10'-0"	9'-6"	8'-3"	7'-3"	6'-10'
"x4			20 PSF	10'-0"	10'-0''	10'-0"	9'-2"	7'-3"	6'-5"
4		V. 1770-201	30 PSF	10'-0"	10'-0"	9'-4''	7'-6''	6'-0''	5'-3"
		20 PSF	40 PSF	10'-0"	10'-0"	8'-1"	6'-6"	5'-1"	4'-6"
			50 PSF	10'-0''	9'-2"	7'-3"	5'-9"	4'-7"	4'-1"
	15'-0"		20 PSF	10'-0''	10'-0"	7'-10''	5'-2"	Na.	70 -
			30 PSF	10'-0"	9'-2"	6'-5"	4'-3''	- 3-	
		30 PSF	40 PSF	10'-0"	8'-0''	5'-7''	-		-
			50 PSF	10'-0"	7'-1"	5'-0''	-		-
			20 PSF	10'-0''	8'-0"	4'-3''	-	-	8
		200 5000	30 PSF	10'-0''	6'-6"	-	-	-	-
		40 PSF	40 PSF	9'-2"	5'-8"	-	-	-	_
			50 PSF	8'-2"	5'-0''	-	_		_

					100	MAX	MUM COLU	MN SPACING	G (FT)	
9	COLUMN TYPE	MAX ROOF SPAN S	GRAVITY/UPLIFT ASD DESIGN LOAD	WIND LOAD	6'-0"	8'-0"	10'-0"	12'-0"	14'-0"	15'-0"
Š		(FT)	(PSF)	(PSF)		ALI	OWABLE PO	ST HEIGHT (FT)	
ų,				20 PSF	10'-0''	10'-0''	10'-0"	7'-11''	5'-4''	4'-3"
				30 PSF	10'-0''	10'-0''	9'-0"	6'-5"	4'-4"	9
			10 PSF	40 PSF	10'-0''	10'-0''	7'-9"	5'-7"	L	-
				50 PSF	10'-0''	9'-5"	7'-0''	5'-0"	-	-
				20 PSF	10'-0''	4'-5"	-		-	-
			20.005	30 PSF	9'-1''	150	-	(=		-
			20 PSF	40 PSF	7'-11''		2	12	-	2
				50 PSF	7'-1"	-	N		124	
		12 '-0"		20 PSF		-		4	11.3	-
				30 PSF	-	-	- 1		71	2)-1
			30 PSF	40 PSF	16	-		- 4	A + 14	7 5
	₽.			50 PSF	~	-	2	12	5	9
	3"x3"x0.093" Post (Unbraced) 10ft		4 0 PSF	20 PSF		-		.=	-	-
v	Sec			30 PSF	7.	-	-	-	-	- 1
0	- de			40 PSF	/:	-	-		-	
7	Ž			50 PSF	V/	-	2	-	#	-
	ost		10 PSF	20 PSF	10'-0''	10'-0''	8'-6"	5'-2"	2:	2
	- A			30 PSF	10'-0''	10'-0''	7'-0''	4'-3''	-	-
ĸ.	60			40 PSF	10'-0''	9'-0''	6'-0''	-	- 4	-
	, v			50 PSF	10'-0''	8'-0''	5'-4''		-47	-
N	, x3		AM	20 PSF	7'-4"	-	2		7-: 7	8
Œ,	m			30 PSF	6'-0"	2	-	- 1		2.1
			20 PSF	40 PSF	5'-2"					-
				50 PSF	4'-7''	-			1	
		15'-0"		20 PSF		-		1	- 7	-
				30 PSF 🥌	15	-	12		7	
			30 PSF	40 PSF	N v	5.00	- 7-		-	5
				50 PSF	. .		-		7	-
				20 PSF	\ -	-	- 1	-	-	-
	\times \vdash	////		30 PSF	-	-	- T		-	-
		a a dia	40 PSF	40 PSF	- 1	-	9.	-	-	-
				50 PSF		-	-	-	_	2

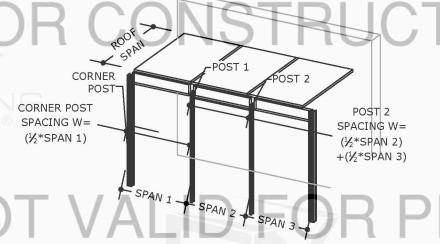
				MAXIMUM COLUMN SPACING (FT)					
COLUMN TYPE	MAX ROOF SPAN S	GRAVITY/UPLIFT ASD DESIGN LOAD	WIND LOAD	6'-0"	8'-0"	10'-0"	12'-0"	14'-0"	15'-0"
,,,,,	(FT)	(PSF)	(PSF)		ALL	OWABLE PO	ST HEIGHT	(FT)	
	100		20 PSF	10'-0"	10'-0"	10'-0''	10'-0"	10'-0"	10'-0'
		10.000	30 PSF	10'-0"	10'-0''	10'-0"	10'-0''	10'-0''	10'-0
		10 PSF	40 PSF	10'-0"	10'-0''	10'-0''	10'-0''	10'-0"	10'-0
		, I I	50 PSF	10'-0"	10'-0''	10'-0"	10'-0''	10'-0"	10'-0
			20 PSF	10'-0"	10'-0''	10'-0"	10'-0''	10'-0''	10'-0
		20.005	30 PSF	10'-0''	10'-0''	10'-0"	10'-0''	10'-0"	10'-0
		20 PSF	40 PSF	10'-0''	10'-0''	10'-0''	10'-0''	10'-0''	10'-0
	12'-0"		50 PSF	10'-0''	10'-0''	10'-0"	10'-0''	10'-0''	9'-4
	12-0		20 PSF	10'-0''	10'-0''	10'-0"	10'-0''	9'-6"	7'-9
		20 000	30 PSF	10'-0''	10'-0''	10'-0"	10'-0"	7'-9"	6'-4
		30 PSF	40 PSF	10'-0''	10'-0''	10'-0"	9'-5"	6'-9"	5'-6
Ħ			50 PSF	10'-0''	10'-0''	10'-0"	8'-5"	6'-0"	5'-0
)1		40 PSF	20 PSF	10'-0''	10'-0''	10'-0"	7'-2"	-	-
peo			30 PSF	10'-0''	10'-0''	10'-0"	5'-10"	-	-
bra			40 PSF	10'-0''	10'-0''	8'-9''	5'-1"		- 20
5			50 PSF	10'-0"	10'-0''	7'-10''	4'-6"		-
ost		10 PSF	20 PSF	10'-0"	10'-0"	10'-0"	10'-0''	10'-0''	10'-0
ğ			30 PSF	10'-0"	10'-0''	10'-0"	10'-0''	10'-0''	10'-0
1/8			40 PSF	10'-0"	10'-0''	10'-0"	10'-0''	10'-0''	10'-0
* *			50 PSF	10'-0"	10'-0''	10'-0"	10'-0''	10'-0''	10'-0
4"x4"x1/8" Post (Unbraced) 10ft			20 PSF	10'-0''	10'-0''	10'-0"	10'-0''	10'-0''	10'-0
		20 BCF	30 PSF	10'-0''	10'-0''	10'-0"	10'-0''	10'-0''	9'-7
		20 PSF	40 PSF	10'-0''	10'-0"	10'-0''	10'-0"	9'-5"	8'-4
	15'-0"	7/45	50 PSF	10'-0''	10'-0"	10'-0''	10'-0"	8'-5"	7'-5
	15 -0		20 PSF	10'-0"	10'-0"	10'-0"	9'-4"	5'-2"	-
-		30 PSF	30 PSF	10'-0''	10'-0''	10'-0"	7'-8''	4'-3"	-
		30 121	40 PSF	10'-0''	10'-0"	10'-0"	6'-8''	1.0	-
			50 PSF	10'-0''	10'-0''	9'-1"	6'-0''	.=	.50
			20 PSF	10'-0''	10'-0''	7'-6''	-	*	8
		40 BSE	30 PSF	10'-0''	10'-0''	6'-2"	-	-	
	//_//	40 PSF	40 PSF	10'-0''	10'-0"	5'-4"	-	-	-
	/ / /-		50 PSF	10'-0"	9'-2"	4'-9"	-	-	

	CONS	
K		1 H
W 1		

SPACE RESERVED FOR CERTIFYING ENGINEER'S DIGITAL OR PHYSICAL SEAL & DATE OF CERTIFICATION

INAL PE SEAL NOT		
LID FOR PERMIT	6i M	
		1

							_		-
					MAXI	мим соци	MN SPACIN	G (FT)	1
COLUMN TYPE	MAX ROOF SPAN S	GRAVITY/UPLIFT ASD DESIGN LOAD	WIND LOAD	6'-0"	8'-0"	10'-0"	12'-0"	14'-0"	15'-0"
	(FT)	(PSF)	(PSF)		ALI	OWABLE PO	ST HEIGHT	(FT)	
			20 PSF	10'-0''	10'-0"	10'-0"	10'-0"	9'-9"	8'-7"
		10 DCE	30 PSF	10'-0''	10'-0''	10'-0"	10'-0"	8'-0"	7'-0"
		10 PSF	40 PSF	10'-0''	10'-0"	10'-0"	8'-9"	6'-11"	6'-1"
			50 PSF	10'-0''	10'-0"	9'-11''	7'-10"	6'-2"	5'-5"
			20 PSF	10'-0''	10'-0"	5'-5"	1	=	1 7
		20 005	30 PSF	10'-0''	8'-8"	4'-5"	-	-	
	. 1	20 PSF	40 PSF	10'-0''	7'-6"		-	2	
		A A	50 PSF	10'-0''	6'-8"			- 1	
	12'-0"		20 PSF	9'-7"		7 - 1			
al .		30 PSF	30 PSF	7'-10"	-	//	- '		-
			40 PSF	6'-9"					-
¥	3"x3" Fluted Post (Unbraced) 10ft ASA 0t		50 PSF	6'-1"	2	-	-	2	120
H			20 PSF	-	_		/		. 1
cec			30 PSF				-		1
bra	< 1	40 PSF	40 PSF	-		1			
5			50 PSF	1		Y -			1.
ost	_		20 PSF	10'-0''	10'-0''	10'-0"	10'-0"	7'-2"	5'-11'
O D			30 PSF	10'-0''	10'-0"	10'-0"	8'-2"	5'-10"	4'-10'
ute		10 PSF	40 PSF	10'-0''	10'-0"	9'-6"	7'-1"	5'-1"	4'-2"
표			50 PSF	10'-0''	10'-0"	8'-6"	6'-4''	4'-6"	7 = 1
ξ2			20 PSF	10'-0''	6'-8"	-	+	-	
m			30 PSF	10'-0''	5'-5"		N - C	7 .	
		20 PSF	40 PSF	10'-0''	4'-9"				:=:
			50 PSF	9'-0"	4'-3"	- 1-1	1 (R)	-	:=:
	15'-0"	, l	20 PSF	4'-6"	7	-21	-	-	2 = 5
			30 PSF	-	=	-	*		-
		30 PSF	40 PSF	_	=	:2	_	-	
	_		50 PSF	-		-	-	-	-
		/ /	20 PSF	-	1	-	V E	9 - 1	-
			30 PSF	-		-	1 -	Ø - I	-0
	V	40 PSF	40 PSF	<i>J</i> -	-	V	7 -		
	-		50 PSF	-	-		-		



POST SPACING
ISOMETRIC

*THIS DETAIL APPLIES TO
BOTH HOST ATTACHED AND
FREE-STANDING ROOF
OVER OPEN STRUCTURES

- 1) 2015 ALUMINUM DESIGN MANUAL, ALLOWABLE STRESS DESIGN METHOD USED IN ALL TABLES.
 2) MAXIMUM ALLOWABLE POST HEIGHT IS 10 FT.
- DEFLECTION LIMIT = L/120.
- COLUMN SPACING IS HALF THE DISTANCE TO THE LEFT ADDED TO HALF THE DISTANCE TO THE RIGHT OF THE BEAM (AVERAGE COLUMN SPACING).
 - STRUCTURES (CERTIFIED BY OTHERS).
 - 2PSF DEAD LOAD USED IN CALCULATIONS
 - POSITIVE AND NEGATIVE DESIGN PRESSURES CALCULATED FOR USE WITH THIS SYSTEM SHALL BE DETERMINED BY OTHERS ON A JOB-SPECIFIC BASIS IN ACCORDANCE WITH ASCE 7-22 OR ASCE 7-16 BASED ON APPLICABLE CODE.

POSTAL ADDRESS: 2234 NORTH FEDERAL HWY #7664 BOCA RATON, FL 33431 ENGINEERINGEXPRESS.COM

STRUCTALL BUILDING SYSTEMS,

23-69317 SCALE: NTS UNLESS NOTE



FREESTANDING & HOST ATTACHED SYSTEM EAVE BEAM SPANS

Beam	Roof Load Max Roof Span (ft)					C	
Type	(psf)	8'-0"	10'-0"	12'-0"	14'-0"	16'-0"	
18	20 PSF	7'-0''	6'-5''	6'-0''	5'-7''	5'-3''	E 🙃
SMB	30 PSF	5'-9"	5'-3''	4'-10''	4'-6''	4'-3''	Bean n (ft)
2"x4" Bea	40 PSF	5'-0''	4'-6''	4'-2''	4'-0''	3'-8''	Max Spai
2"	50 PSF	4'-5''	4'-1''	3'-9''	3'-6''	3'-4''	≥ "

Beam	Roof Load						
Type	(psf)	8'-0"	10'-0"	12'-0"	14'-0"	16'-0"	U
1S	20 PSF	8'-4''	7'-8''	7'-1''	6'-7''	6'-3''	E ÷
SMS	30 PSF	6'-10''	6'-3''	5'-9''	5'-5''	5'-1''	Bean n (ft)
2"x5" g	40 PSF	6'-0''	5'-5''	5'-0''	4'-8''	4'-5''	Max Spai
2	50 PSF	5'-4''	4'-10''	4'-6''	4'-2"	4'-0''	Σ 0

NIOT VALID

Beam	Roof Load	Max Roof Span (ft)					
Type	(psf)	8'-0"	10'-0"	12'-0"	14'-0"	16'-0"	
SI	20 PSF	10'-7"	9'-8"	9'-0"	8'-4''	7'-11"	m ()
SMS	30 PSF	8'-8''	7'-11"	7'-3"	6'-10''	6'-5''	Bean η (ft)
2"x6" Bea	40 PSF	7'-6''	6'-10''	6'-4''	5'-11''	5'-7''	/lax Spai
2	50 PSF	6'-8"	6'-1"	5'-8''	5'-3''	5'-0''	N S

	Roof Load	<u> </u>	Max Roof Span (ft)				
Type	(psf)	8'-0"	10'-0"	12'-0"	14'-0"	16'-0"	
1S	20 PSF	10'-11''	10'-0"	9'-2''	8'-7''	8'-1"	am ft)
' SMS am	30 PSF	8'-11''	8'-1"	7'-6''	7'-0''	6'-7''	Bea
2"x7" Bea	40 PSF	7'-8''	7'-0''	6'-6''	6'-1''	5'-9''	Aax
2'	50 PSF	6'-11''	6'-3''	5'-10''	5'-5"	5'-2"	≥ °

SPACE RESERVED FOR CERTIFYING ENGINEER'S DIGITAL OR PHYSICAL SEAL & DATE OF CERTIFICATION

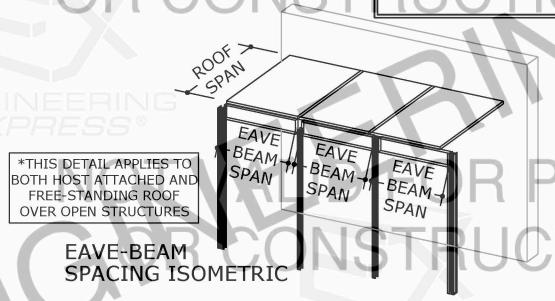
VALID FOR PERMIT

POSTAL ADDRESS: 2234 NORTH FEDERAL HWY #7664 BOCA RATON, FL 33431 ENGINEERINGEXPRESS.COM

STRUCTALL BUILDING SYSTEMS,

23-69317

SCALE: NTS UNLESS NOTE



- TABLE NOTES:

 1) 2015 ALUMINUM DESIGN MANUAL, ALLOWABLE STRESS DESIGN METHOD USED IN ALL TABLES.

 2) DEFLECTION LIMIT = L/120

 3) 2PSF DEAD LOAD CONSIDERED IN CALCULATIONS.

 4) POSITIVE AND NEGATIVE DESIGN PRESSURES CALCULATED FOR USE WITH THIS SYSTEM SHALL BE DETERMINED BY OTHERS ON A JOB-SPECIFIC BASIS IN ACCORDANCE WITH ASCE 7-22 OR ASCE 7-16

Beam	Roof Load	Ŋ	K F				
Type	(psf)	8'-0"	10'-0"	12'-0"	14'-0"	16'-0"	10
IIS	20 PSF	15'-0''	15'-0''	14'-6''	13'-6''	12'-9''	E z
SMS	30 PSF	14'-0''	12'-9''	11'-10''	11'-1''	10'-5''	Bear n (ft)
x8" Be	40 PSF	12'-1''	11'-1''	10'-3''	9'-7''	9'-0''	Мах Ѕраі
X N	50 PSF	10'-10''	9'-11''	9'-2''	8'-7''	8'-1''	Σ 6

Beam	Roof Load		RF				
Type	(psf)	8'-0"	10'-0"	12'-0"	14'-0"	16'-0"	
NS N	20 PSF	15'-0''	15'-0"	15'-0"	15'-0''	14'-2''	Ē
' SMS am	30 PSF	15'-0"	14'-2''	13'-1"	12'-3''	11'-7''	Beam n (ft)
2"x9" Bea	40 PSF	13'-5"	12'-3"	11'-4''	10'-8''	10'-0''	Max Spai
2"	50 PSF	12'-0''	11'-0''	10'-2"	9'-6"	9'-0''	≥ "

FREESTANDIN	IG & HOST	ATTACHED
SYSTEM E	AVE BEAM	SPANS

	1
	 DIGITAL SEAL NOTICE:

U.U	
2"x10" SELF	
MATING	
BEAMS	ĺ

Beam	Roof Load Max Roof Span (ft)						
Type	(psf)	8'-0"	10'-0"	12'-0"	14'-0"	16'-0"	
ge	20 PSF	15'-0"	15'-0"	15'-0"	15'-0''	15'-0''	E E
Edge	30 PSF	15'-0''	15'-0''	14'-6''	13'-7"	12'-9"	Bear n (ft)
x8" Bea	40 PSF	14'-10'	13'-7''	12'-6''	11'-9''	11'-1''	Max B Span
2".	50 PSF	13'-3''	12'-1''	11'-3''	10'-6''	9'-7''	≥ 0

2"x8" **EDGE BEAM**

Beam	Roof Load		/lax Ro	of Spa	an (ft	Ъг	
Туре	(psf)	8'-0"	10'-0"	12'-0"	14'-0"	16'-0"	
90	20 PSF	12'-7''	11'-6''	10'-8"	10'-0"	9'-5''	E 🕤
Edge am	30 P S F	10'-3"	9'-5"	8'-8''	8'-1''	7'-8''	Bear 7 (ft)
2"x5" Bea	40 PSF	8'-11''	8'-1"	7'-6''	7'-0''	6'-8''	Max B Span
2"	50 PSF	8'-0''	7'-3''	6'-9''	6'-3''	6'-0''	≥ 0

Beam

Type

2"x7" Edge Beam

Roof Load

(psf)

20 PSF

30 PSF

40 PSF

50 PSF

Max Roof Span (ft)

9'-8"

10'-0"

' 10'-5''

8'-0"

11'-5'

15'-0" 15'-0'

14'-9" 13'-5"

12'-9" 11'-8"

12'-0" 14'-0" 16'-0"

15'-0" 14'-3" 13'-5"

12'-5" 11'-8" 11'-0"

10'-9" 10'-1" 9'-6"

9'-0''

8'-6"

2"x5"	EDGE
BE	AM

	MIT
Max Beam Span (ft)	N
	7" EDGE

"マフ"	EDGE
	LDGL
BF	ΔΜ

	Beam	eam Roof Load Max Roof Span (ft))		
	Type	(psf)	8'-0"	10'-0"	12'-0"	14'-0"	16'-0"		
	٤	20 PSF	14'-2''	13'-0"	12'-0''	11'-2''	10'-7''	Ε	Æ)
	C-Beam	30 PSF	11'-7"	10'-7''	9'-9''	9'-2"	8'-7''	Bea	<u>ب</u>
		40 PSF	10'-0"	9'-2''	8'-5"	7'-11"	7'-5"	ах	Span
Į	6.7	50 PSF	9'-0"	8'-2''	7'-7''	7'-1''	6'-8''	Σ	U)

6.7 C-BEAM

	Beam	Roof Load	N	Max Roof Span (ft)					Max Roof Span (ft)				
	Type	(psf)	8'-0"	10'-0"	12'-0"	14'-0"	16'-0"		F				
ĺ	pa m	20 PSF	15'-0"	15'-0''	15'-0"	15'-0''	15'-0"	_	⊕				
1	orce Bea	30 PSF	1 5'-0''	15'-0''	15'-0''	14'-2"	13'-5"	Bea	£				
I	2"x7' sinfor Ige Be	40 PSF	15'-0"	14'-2"	13'-2"	12'-4"	11'-7"		Spai				
	Re	50 PSF	13'-11''	12'-8''	11'-9''	11'-0"	10'-4''	Σ	0)				

3	И	ľ
$\bigcup_{i=1}^{n}$	N	
Je	2"x7"	

EDGE BEAM

Span (ic)	N		
	2"x7"		
REI	NFORC	CED	

	Beam	Roof Load	Max Roof Span (ft)						
Ī	Type	(psf)	8'-0"	10'-0"	12'-0"	14'-0"	16'-0"		
١	#	20 PSF	7'-7''	6'-11''	6'-5''	6'-0''	5'-8''	Е	
	" Tilt am	30 PSF	6'-2''	5'-8''	5'-3"	4'-10''	4'-7''	Bea	n (ft.)
	"x4" Bea	40 PSF	5'-4"	4'-10''	4'-6''	4'-3"	4'-0''	Max	Spai
	2"	50 PSF	4'-9"	4'-4''	4'-0''	3'-9"	3'-7"	Σ	0)

4" TILT BEAM

POSTAL ADDRESS:
2234 NORTH FEDERAL HWY #7
BOCA RATON, FL 33431
ENGINEERINGEXPRESS.CON

23-69317 SCALE: NTS UNLESS NOTE



